Development and Design of Cost-Effective, Real-Time Implementable Sediment and Contaminant Release Controls

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Background

- High priority on treatment of contaminated storm water and sediment from PGDP
- Industry-standard engineering approaches have been considered
- Involve
 - Large expenditures of capital
 - -Long implementation timeframe

Objectives

- Evaluate the adequacy and expected performance of existing storm water controls
- Develop alternative storm water and sediment treatment systems
- Assess and provide recommendations for identified storm water and sediment remedial options
 - -cost effective
 - -able to be implemented in a short timeframe

Assessment of Current Conditions -Watershed Characteristics

 Outfall 011 - Area Calculated Percent impervious area. Flow conveyance for all watershed - Storm water inlets - Associated piping netwo - Open channel waterwa

Assessment of Current Conditions -Watershed Characteristics



 Assessment of Current Conditions – Modeling Current Conditions
 SEDCAD version 4.0 (Warner et al. 1998)

- Curve Numbers
 - 92 for impervious areas, buildings, paved and gravel areas
 - 79 for grassed areas (hydrologic soil group C)
- Time of concentration 0.126
- Unit hydrograph response functions assigned
 - -Fast for impervious areas
 - Medium for grassed areas

Modeling Current Conditions

- Erosion parameters similarly assigned
- Predominant soil series are:
 - Henry-Grenada-Calloway
- K-factor (erodibility) 0.28



Modeling Current Conditions

- Representative slope lengths and gradients
 - Impervious areas
 - Slope length 150 ft.
 - Slope gradient 1%
 - Grassed areas
 - Slope length 100 ft.
 - Slope gradient 4%
- C-factor (cover factor)
 Impervious areas 0.02
 Grassed areas 0.013



Assessment of Current Conditions -Modeling Results

- Predicted sediment load and concentrations are low for all three outfalls
 - -high density of impervious areas
 - -well established grass cover
- Storms (0.5 to 3in) Outfall 015
 - -peak sediment concentrations ranged from 450 600 mg/L
 - -peak runoff 3.8 99.8 cfs
 - -runoff volume 0.37 ac-ft. to 9.58 ac-ft.

Alternative Storm Water and Sediment Control Systems

 Retention Pond Performance – Design Storm Basis

- -Outfall 011
- -Outfall 015
- -Outfall 008

Retention Pond Performance – Annual Basis

Alternative Secondary Treatment Systems







Retention Pond & Embankment Design

| Attribute | Outfall 011 | Outfall 015 | Outfall 008 |
|---------------------------------|-------------|-------------|-------------|
| Embankment Crest Elevation (ft) | 377.5 | 365 | 363 |
| Emergency Spillway | | | |
| Invert (ft) | 377 | 363 | 361 |
| Width (ft) | 60 | 25 | 25 |
| Drop Inlet | | | |
| Invert (ft) | 375 | 361 | 359 |
| Diameter (in) | 36 | 36 | 36 |
| Pond Capacity (ac-ft) | | | |
| @ Top of Dam | 6.67 | 3.51 | 3.03 |
| @ Emergency Spillway | 5.92 | 2.03 | 1.70 |
| @ Principle Spillway | 3.66 | 0.97 | 0.92 |
| 100yr 24hr Freeboard (ft) | 0.0 | 0.17 | Overflows |

Retention Pond Performance – Design Storm Basis: Outfall 011

- Initial condition empty at beginning of storm event
- Runoff contained in the pond pumped to the treatment system located near Outfall 010
- Completely contain a 2-in rainfall event (3.43 ac-ft)
- 3-in storm
 - reduce the peak flow from 63 to 5 cfs
 - ~100 % sediment trapping

 Performance of Outfall's 011 pond is predicted to be excellent; essentially trapping all entering sediment for storm events less than 4 inches

Retention Pond Performance – Design Storm Basis: Outfall 015

- Storage volume for Pond 015 much smaller than Pond 011
- Watershed area is greater: 55.5 vs. 33.3 acres
- Without excavation and starting empty, Pond 015 completely contain ¾-in storm
- Predicted sediment trap efficiency
 - 1.5-in storm 98.2 %
 - 2.0-in storm 85.5 %
 - 3.0-in storm- 72.3 %

Retention Pond Performance – Design Storm Basis: Outfall 008

- Watershed area of 113.6 acres exceeds Outfall 015 by more than a factor of two
- The pond capacity, below the principle spillway, is 0.92 ac-ft, ~ the same as Outfall 015
- Contain a ½-in storm without discharging
- Predicted sediment trapping efficiencies
 - **1.0-in**, **96.7%**
 - 1.5-in, 77.2%
 - **2.0-in**, 67.6%

Retention Pond Performance – Annual Basis

- Analyzed Paducah airport daily precipitation data 1971 to 2000
- Cumulative rainfall curve

| Rainfall (in) | % |
|---------------|----|
| 0.5 | 24 |
| 0.75 | 40 |
| 1.0 | 52 |
| 1.25 | 62 |
| 1.5 | 70 |
| 2.0 | 82 |
| 3.0 | 92 |

| | | | Outfall 011 | Outfall 015 | Outfall 008 |
|---------------|-------------|-------------|-------------|-------------|-------------|
| Rainfall | Rainfall | | Runoff * | Runoff * | Runoff * |
| (in) | midpoint | Probability | (%) | (%) | (%) |
| 0.10-0.25 | 0.175 | 5.32 | 100 | 100 | 100 |
| 0.25-0.50 | 0.375 | 15.02 | 100 | 100 | 100 |
| 0.50-0.75 | 0.625 | 15.94 | 100 | 100 | 66 |
| 0.75-1.00 | 0.875 | 11.67 | 100 | 73 | 32 |
| 1.00-1.25 | 1.125 | 10.23 | 100 | 46 | 20 |
| 1.25-1.50 | 1.375 | 7.93 | 100 | 32 | 14 |
| 1.50-1.75 | 1.625 | 5.61 | 100 | 25 | 11 |
| 1.75-2.00 | 1.875 | 6.09 | 100 | 20 | 9 |
| 2.00-2.25 | 2.125 | 3.88 | 100 | 16 | 7 |
| 2.25-2.50 | 2.375 | 2.09 | 84 | 14 | 6 |
| 2.50-2.75 | 2.625 | 1.77 | 73 | 12 | 6 |
| 2.75-3.00 | 2.875 | 2.33 | 64 | 11 | 5 |
| 3.00-3.25 | 3.125 | 1.00 | 58 | 10 | 4 |
| 3.25-3.50 | 3.375 | 1.14 | 52 | 9 | 4 |
| 3.50-3.75 | 3.625 | 0.73 | 48 | 8 | 4 |
| 3.75-4.00 | 3.875 | 1.83 | 44 | 7 | 3 |
| 4.00-4.50 | 4.25 | 0.84 | 39 | 6 | 3 |
| 4.50-5.00 | 4.75 | 0.95 | 34 | 5 | 3 |
| 5.00-5.50 | 5.25 | 0.71 | 30 | 5 | 2 |
| Annual contai | nment in po | onds | 83.1% | 34.7% | 20.2% |

* Runoff volume contained in ponds

Retention Pond Performance – Annual Runoff Volume Treated by Secondary System

| Outfall | Largest Storm | Outfall | Annual Runoff Volume (%) |
|---------|------------------|---------|-----------------------------------|
| 011 | 2 | 011 | 83.1 |
| 015 | 3/4 | 015 | 34.7 |
| | | 800 | 20.2 |
| 800 | 1/2 | | |

Alternative Secondary Treatment Systems

•Designs : -irrigation (outfall 015) evapotranspiration (ET) - drip - micro-sprayers evapotranspiration-infiltration (ET-I) - drip - micro-sprayers -weep berm (outfall 008)

Design Alternative: ET

Advantage of restricting application rate to match ET rate:

 vast majority of water applied will be treated without the potential for groundwater contamination

Disadvantage

- -slower dewatering rate of pond
- -primarily applicable April October

Design Alternative: ET-I

Advantages: evapotranspiration-infiltration system:

- ability to have a higher applications rate
- longer duration of application -> treatment of a greater volume of water compared to the evapotranspiration method

 Disadvantage: portion of the applied water may migrate to groundwater

Evapotranspiration Method

Figure 7: Average Precipitation-Evapotranspiration Difference by Month



Evapotranspiration Considerations

Daily ET

- > 0.10 in (April Oct)
- > 0.16 in (May Sept)
- > 0.23 in (June Aug)
- ET applicable ~ 7 months/yr



Drip Irrigation System



PR - pressure regulator (10 psi)

Evapotranspiration Method – Drip Irrigation System

- Dewatering Time (daily ET 0.11)
 - 21 days (5 ac)
 - 4 days (25 ac)
- Dewatering Time (daily ET 0.22) June August
 - 10 days (5 ac)
 - 2 days (25 ac)



Infiltration Assumptions

- Soil infiltration rate based soil texture
- Steady state infiltration rate (hydrologic soil group 'C') 0.05 to 0.15 in/hr
- Due to macropores, the infiltration rate may be substantially higher.
- Initial infiltration rate 0.4 to 0.5 in/hr and short duration irrigation application rates can exceed 0.6 in/hr without runoff.

Evapotranspiration/Infiltration Method – Drip Irrigation System

Assumed

- steady state infiltration rate of 0.1 in/hr
- 10-hour irrigation duration
- Dewatering Time
 - 2 days (5 ac)

Micro-sprinkler Irrigation System

• Micro-sprinklers:

- small rotating spray heads
- radius ~ 15 ft
- 1 gpm
- Close to the ground
- Limited exposure to drift
- Evaporation rate of spray ~ 20% of application rate
- Spatial coverage is better than drip ET more uniform
- Higher irrigation application rate than drip operating times are reduced

Evapotranspiration Method – Micro-sprinkler Irrigation System

- Head-to-head coverage spacing: 15-ft spacing between sprayers
- ~ 200 micro-sprayers /ac
- Application rate 0.43 in/hr
- Operation time/zone:
 - ET rate of 0.11 inch/day: 15 minutes/day
 - ET rate of 0.22 inch/day: 30 minutes/day.

5-Acre Micro Sprinkler Irrigation Design



Evapotranspiration Method – Micro-sprayer Irrigation System

- Operate on a pulse irrigation method
- 1.0 inch daily infiltration (1.3 in/day)
- 0.54 ac-ft/day applied
- Each 1-ac zone operate ~ 3 hrs/day
- Total operating time (5 zones): 15 hours/day
- Time to dewater Pond 015:
 - 8 days (5 ac)
 - ~1 ¾ days (25 ac)



Evapotranspiration/Infiltration Method – Micro-sprayer Irrigation System

- Operate on a pulse irrigation method
- 5-ac site
 - 1.0 inch daily infiltration (1.3 in/day)
 - 0.54 ac-ft/day applied
- Each 1-ac zone operate ~ 3 hrs/day
- Total operating time (5 zones): 15 hours/day

Time to dewater Pond 015: ~ 1 3/4 days (5 ac)

Combined Weep Berm – Grass Filter

- A weep berm simply an earthen berm that temporarily detains water that is slowly and passively discharged through multiple pipes, to the down-gradient grass filter.
- Low cost, easily constructed, and highly effective
- Further treatment and infiltration occurs along the grass filter prior to any residual runoff re-entering Outfall 008's retention pond.
- Works synergistically with the down-gradient riparian zone and blends into the natural landscape
- A combination weep berm-grass filter reduces sediment concentration







Combined Weep Berm – Grass Filter

Weep Berm Design Parameters

- length 450 ft
- height 2 ft
- storage capacity 0.275 ac-ft
- 1-in PVC pipes at 10 ft spacing and 1 ft invert
- pumping rate from Pond 008 450 gpm
- pump operating time 6 hr/day
- Dewatering time for Pond 008 ~ 2 days

Combined Weep Berm – Grass Filter

• Grass Filter Design Parameters

- length 250 ft
- slope 4 %
- steady-state infiltration rate 0.1 in/hr
- grass existing vegetation

Weep Berm – Grass Filter Performance

- Storm 0.7 in
- Weep berm steady state stage 1 ¾ ft
- Freeboard ¼ ft
- Sediment trap efficiency of weep berm additional 36%
- Peak effluent 88 mg/L
- Sediment trap efficiency of grass filter ~ 100 %
- Peak effluent 2 mg/L

Findings - Sediment Trap Efficiency of Ponds

| Outfall | Sediment Trap Efficiency (%) | Storm Size (in) |
|---------|---------------------------------|--------------------|
| 011 | 99.7 | 4 |
| 015 | 72.3 | 3 |
| 800 | 67.6 | 2 |

Findings - Annual Runoff Volume Treated by Secondary System

| Outfall | % |
|---------|------|
| 011 | 83.1 |
| 015 | 34.7 |
| 008 | 20.2 |

Findings – Dewatering Time Pond 015

| | Dewatering Pond 015 (days) | | |
|-----------------|----------------------------|-------|--|
| Treatment | 5 ac | 25 ac | |
| System | | | |
| ET | | | |
| Drip | 21 | 4 | |
| Micro | 8 | 1 3/4 | |
| ET-Infiltration | | | |
| Drip | 2 | n/a | |
| Micro | 1 3/4 | n/a | |

General Findings – Weep Berm-Grass Filter

➢Pond 008

~ 100% sediment retention